

CONCEPTUAL HYDROLOGY STUDY

For

**VTTM No. 67505
22255 Mulholland
22241–22255 Mulholland Drive,
Woodland Hills, CA 91364**

Psomas Project No.: 1HAR262500
September 27, 2018

Prepared for:

**Harridge San Feliciano LLC
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Los Angeles, CA 90048
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Prepared by:

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A handwritten signature in blue ink, appearing to read "Patrick Ho".

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1.0 PROJECT SUMMARY

1.1 PROJECT PURPOSE AND SCOPE

The purpose of this study is to demonstrate that the proposed project site can be designed to provide adequate flood protection for on-site improvements without adversely impacting existing off-site drainage facilities or adjacent properties through the use of storm drain systems.

Psomas has been retained by Harridge San Feliciano LLC to prepare a conceptual hydrology report for the proposed Mulholland Project. The 6.2-acre project site is located northeast of the intersection of Mulholland Drive and San Feliciano Drive in the City of Woodland Hills. The site was previously a privately-owned ranch property, and the proposed improvements include tract development per VTTM No. 67505. The development, as prepared by Westcon Engineering, Inc dated August 21, 2009, consists of 19 detached single-family residences and associated improvements. (See Hydrology Map, Section 4.0)

1.2 EXISTING AND PROPOSED DRAINAGE CONDITIONS

EXISTING DRAINAGE CONDITIONS

Storm runoff from the entire project site is currently conveyed by overland flow into the existing Los Angeles County Flood Control District (LACFCD) storm drain system in San Feliciano Drive (Project No. 5229). Portions of the existing 81-inch County storm drain system and its fifteen-foot easement currently run through the southwest portion of the project site.

The project area is currently undeveloped with varying slopes, ranging from 2% to 50%. The percentage imperviousness is estimated at 15%. Storm runoff travel from the eastern, southern, and western boundaries to the middle of the site and flow north towards San Feliciano Drive. The easterly boundary of the site is bordered by the Girard Reservoir.

An existing 30-inch City of Los Angeles storm drain system (Plan D-17484) parallel to the centerline of Mulholland Drive collects runoff from the south side of the road. Separately, the runoff from the north side of Mulholland Drive and the undeveloped area along the public street outside the property boundary is currently conveyed by overland flow into the project area which is ultimately conveyed to San Feliciano Drive into the existing 81-inch county system. This additional tributary area from Mulholland Drive and Girard Reservoir, results in an increase of 3.9-acre off-site area that is contributing run-on to the project property. Therefore, the total tributary area of 10.1 acres is to be used as the basis of the hydrologic analysis.

The existing condition is identified in the Existing Hydrology Map located in Section 4.0. Existing condition hydrology results for the 25-year and 50-year storm event are summarized in Table 1 below.

Table 1: Existing Condition Hydrology Summary

Sub Area	Area (ac)	25- year and 50-year Storm Events			
		Time of Conc. (min)	Q ₂₅ (cfs)	Time of Conc. (min)	Q ₅₀ (cfs)
1A	10.1	10	25.97	9	31.54

PROPOSED DRAINAGE CONDITIONS

The project site will be developed to accommodate 19 detached single family residences. Associated site improvements include curb and sidewalks, common area landscaped for new construction, sidewalk dedication, drainage facilities, and paving and grading for access roads, driveways, and private street and parking spaces.

The disturbed area in the proposed condition is approximately 2.8 acres. For Units 5 through 19 located at the northerly portion of the site, runoff from the disturbed area is collected into proposed catch basins at the low point of the private street. These catch basins are part of the proposed on-site storm drain system that is designed to convey runoff north to be discharged into the existing County storm drain system located within the property. For Units 1 through 4 located at the southerly portion of the site, runoff is collected into a separate on-site storm drain system where the pipe is designed to daylight at the northeast corner of the disturbed area to its natural drainage path. The remaining 7.3-acre undisturbed area will follow the existing natural flow path through onsite native vegetation north of the Girard Reservoir, into the existing County storm drain system on San Feliciano Drive.

The proposed condition is identified in the Proposed Hydrology Map located in Section 4.0. Proposed condition hydrology results for the 25-year and 50-year storm event are summarized in Table 2 below.

Table 2: Proposed Condition Hydrology Summary

Area	Area (ac)	25- year and 50-year Storm Events			
		Time of Conc. (min)	Q ₂₅ (cfs)	Time of Conc. (min)	Q ₅₀ (cfs)
1A	10.1	14	26.08	9	31.57

1.3 HYDROLOGIC ANALYSIS

The methodology described in the Los Angeles County Department of Public Works Hydrology Manual was used to compute storm water runoff rates from the project site to the existing storm drains. A hydrology study was tabulated and compared between the existing conditions and proposed conditions. Peak flows in excess of the proposed hydrologic study will be detained on-site.

The hydrologic methods used in this study were based on procedures described in the Los Angeles County Department of Public Works Hydrology Manual. The method used is the “Rational Method” (for sub-area time of concentration computation).

The *HydroCalc Calculator* program, developed by County of Los Angeles, was used to calculate the time of concentration and peak runoff flow rate for the existing and proposed conditions. Results of the calculations are included in Section 2.0 for reference. In accordance with LACDPW requirements, since the northerly portion of the site is designed with a sump condition of the private street, the 50-year storm event was used as the main design storm in this analysis. 25-year storm event is also included in the tables for reference.

The Mulholland project is located in City of Woodland Hills in the Canoga Park quadrant of Isohyetal Map Figure LACDPW 1-H1.26. The 50-year 24-hour rainfall Isohyet nearest the project area is 7.70, while the 25-year 24-hour rainfall Isohyet in the project area is 6.76. The project falls within two LACDPW-defined soil classification types: 002 and 004. Because the project site consists of predominantly soil classification 002, calculations were based on the 002 soil type. The overall property is 6.2 acres. The proposed development consists of a disturbed area of 2.8 acres. The total tributary area with off-site run-on 10.1 acres.

1.4 HYDROLOGIC ANALYSIS CONCLUSION

The hydrology calculations demonstrate that the proposed site can be protected from flooding through the combination of existing off-site drainage facilities and proposed on-site storm drains. The following table summarizes the calculated flow rates and allowable discharge rates from each sub-area.

Table 3 – Existing vs. Proposed Condition Hydrology Comparison

25-Year Storm Event						
Drainage Area	Area (ac)			Q ₂₅ (cfs)		
	Exist.	Prop.	Diff.	Exist.	Prop.	Diff.
1A	10.1	10.1	0.0	25.97	26.08	+0.11

50-Year Storm Event						
Drainage Area	Area (ac)			Q ₅₀ (cfs)		
	Exist.	Prop.	Diff.	Exist.	Prop.	Diff.
1A	10.1	10.1	0.0	31.54	31.57	+0.03

Under both storm events, while the percent imperviousness increases as a result of the proposed improvements, the peak flow generated in the proposed conditions remain relatively the same as that of the existing condition. In addition, LID stormwater volume is designed to be retained on-site as part of the water quality design, thereby further reducing the downstream discharge. Therefore, detention will not be necessary for the proposed improvements.

1.5 LID WATER QUALITY DESIGN

The LID requirements, approved by the Regional Water Quality Control Board, requires the treatment of the peak mitigation flow rate or volume of runoff produced by a 0.75" 24-hr storm event or 85th percentile 24-hr storm event: whichever is greater. The 85th percentile 24-hr storm event generates a rainfall of 1.1 inch for the project site, which is the greater of the two scenarios, and therefore will be used in the peak mitigation flow/volume calculations.

The LID calculation methodology was used to calculate the required treatment flows and volumes for the proposed discharge point from the site. LID Calculations are provided in Section 3.0. The results are summarized in the tables below.

Table 4: LID Calculation Results

85 th Percentile Storm Event			
Sub-Area	Area [ac]	Q _{PM} [cfs]	V _M [ft ³]
1A	2.8	0.62	9,133

The project site is designed to retain a minimum of 9,133 cubic feet to be in satisfaction with the LID requirements. BMPs will be designed to comply with the infiltration feasibility recommendations as determined by the geotechnical engineer. If infiltration is not feasible, biofiltration BMPs will be considered during the design phase.

1.6 LIMITATIONS

- This report was prepared to comply with the guidelines established by the City of Los Angeles and their representatives.
- Usage of this report is limited to address the purpose and scope previously defined by the project owner. Psomas shall not be held responsible for any unauthorized application of this report and the contents herein.
- The opinions presented in this report have been derived in accordance with current standards of civil engineering practice. No other warranty is expressed or implied.

Section 2.0

RATIONAL METHOD Tc CALCULATIONS

Peak Flow Hydrologic Analysis

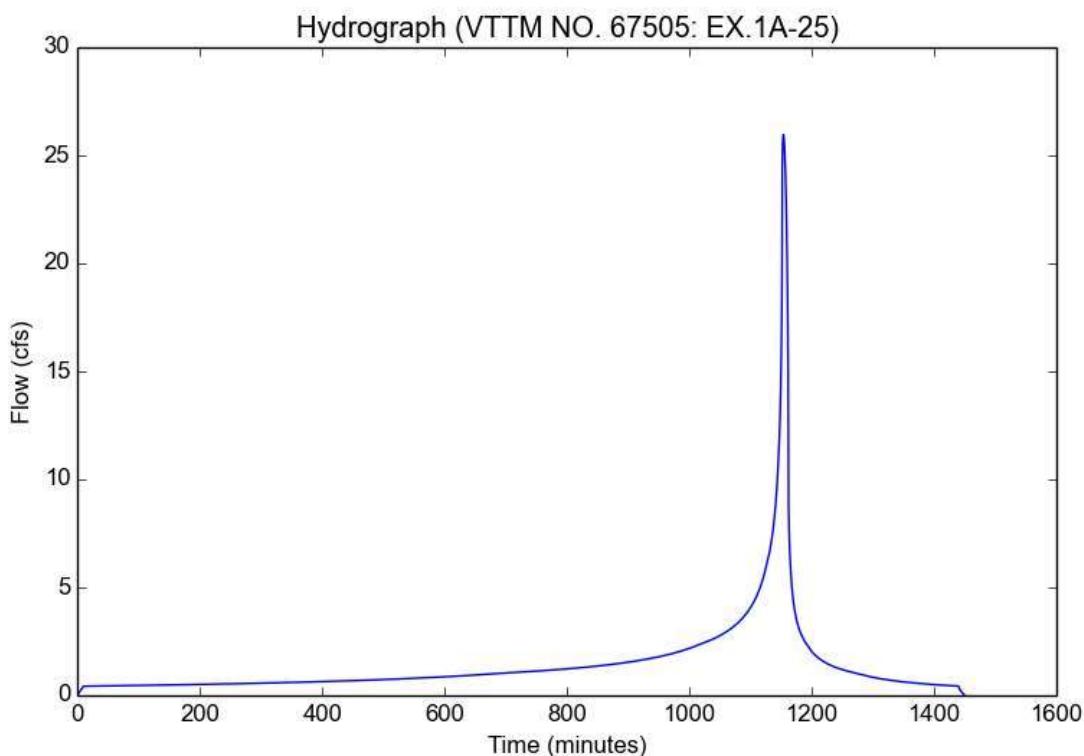
File location: W:/1HAR262500/ENGR/DESIGN/HYDR/EXISTING/VTTM NO. 67505 - EX.1A-25.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM NO. 67505
Subarea ID	EX.1A-25
Area (ac)	10.1
Flow Path Length (ft)	1400.0
Flow Path Slope (vft/hft)	0.03428
50-yr Rainfall Depth (in)	7.7
Percent Impervious	0.15
Soil Type	2
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	6.7606
Peak Intensity (in/hr)	2.9121
Undeveloped Runoff Coefficient (Cu)	0.8798
Developed Runoff Coefficient (Cd)	0.8828
Time of Concentration (min)	10.0
Clear Peak Flow Rate (cfs)	25.9654
Burned Peak Flow Rate (cfs)	25.9654
24-Hr Clear Runoff Volume (ac-ft)	2.8999
24-Hr Clear Runoff Volume (cu-ft)	126318.5462



Peak Flow Hydrologic Analysis

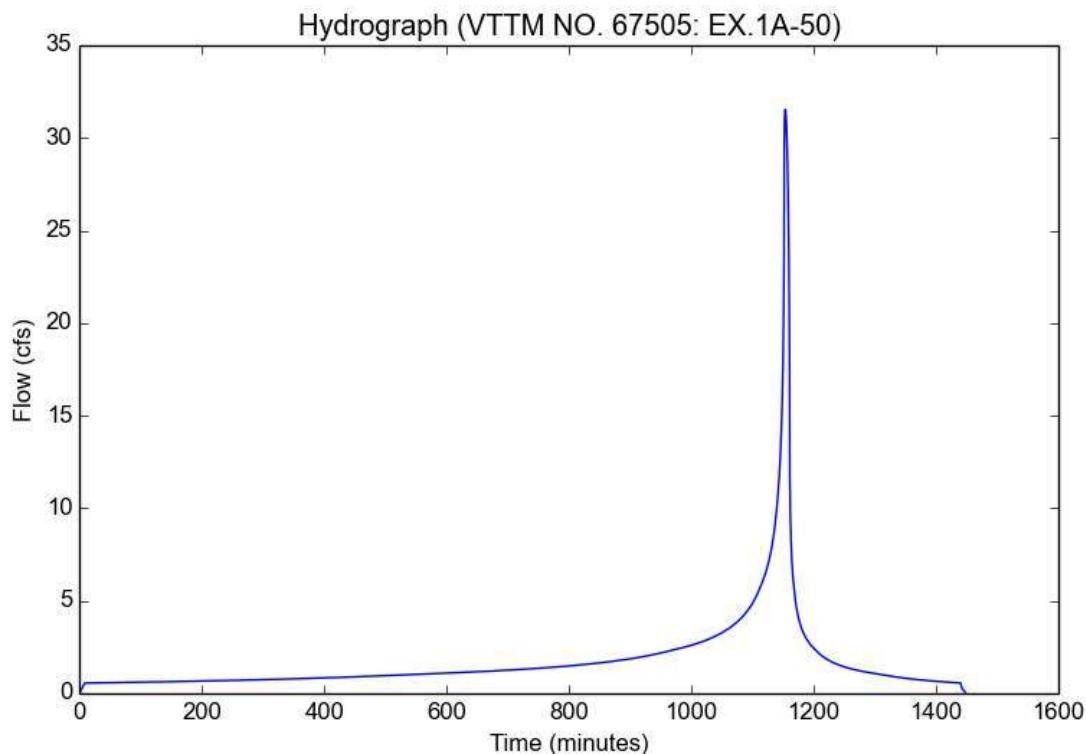
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM NO. 67505
Subarea ID	EX.1A-50
Area (ac)	10.1
Flow Path Length (ft)	1400.0
Flow Path Slope (vft/hft)	0.03428
50-yr Rainfall Depth (in)	7.7
Percent Impervious	0.15
Soil Type	2
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	7.7
Peak Intensity (in/hr)	3.4851
Undeveloped Runoff Coefficient (Cu)	0.8954
Developed Runoff Coefficient (Cd)	0.8961
Time of Concentration (min)	9.0
Clear Peak Flow Rate (cfs)	31.5415
Burned Peak Flow Rate (cfs)	31.5415
24-Hr Clear Runoff Volume (ac-ft)	3.5206
24-Hr Clear Runoff Volume (cu-ft)	153356.64



Peak Flow Hydrologic Analysis

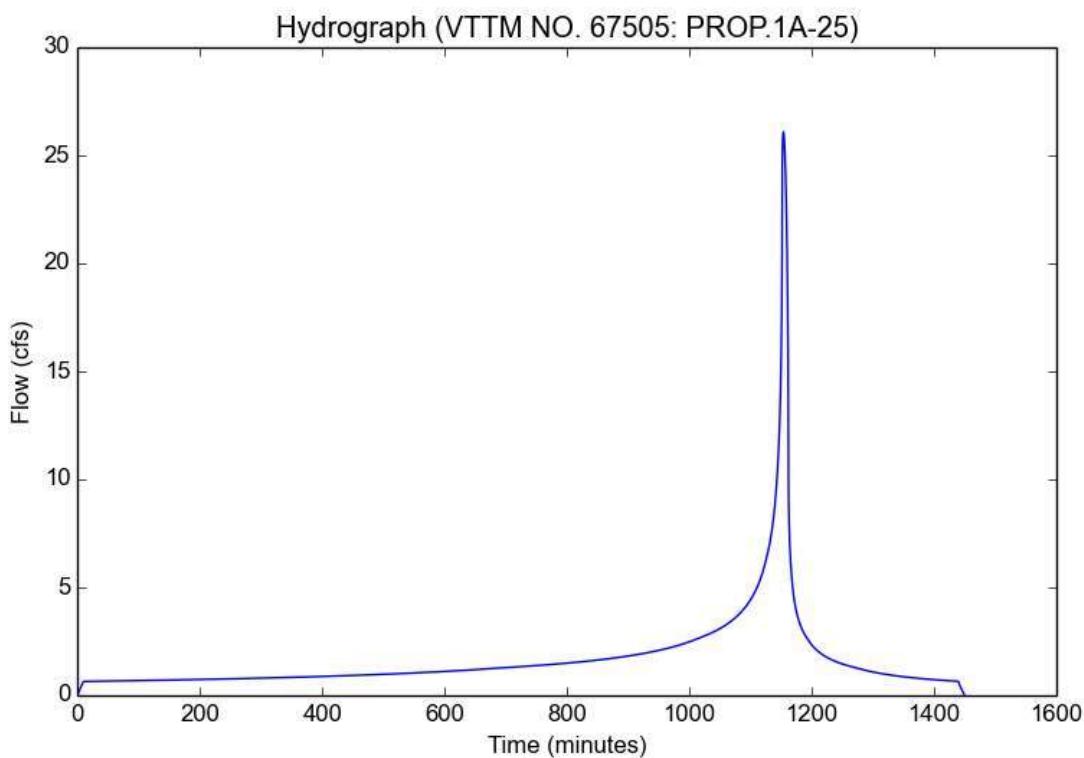
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM NO. 67505
Subarea ID	PROP.1A-25
Area (ac)	10.1
Flow Path Length (ft)	1151.0
Flow Path Slope (vft/hft)	0.015
50-yr Rainfall Depth (in)	7.7
Percent Impervious	0.35
Soil Type	2
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	6.7606
Peak Intensity (in/hr)	2.9121
Undeveloped Runoff Coefficient (Cu)	0.8798
Developed Runoff Coefficient (Cd)	0.8869
Time of Concentration (min)	10.0
Clear Peak Flow Rate (cfs)	26.0843
Burned Peak Flow Rate (cfs)	26.0843
24-Hr Clear Runoff Volume (ac-ft)	3.4126
24-Hr Clear Runoff Volume (cu-ft)	148651.6301



Peak Flow Hydrologic Analysis

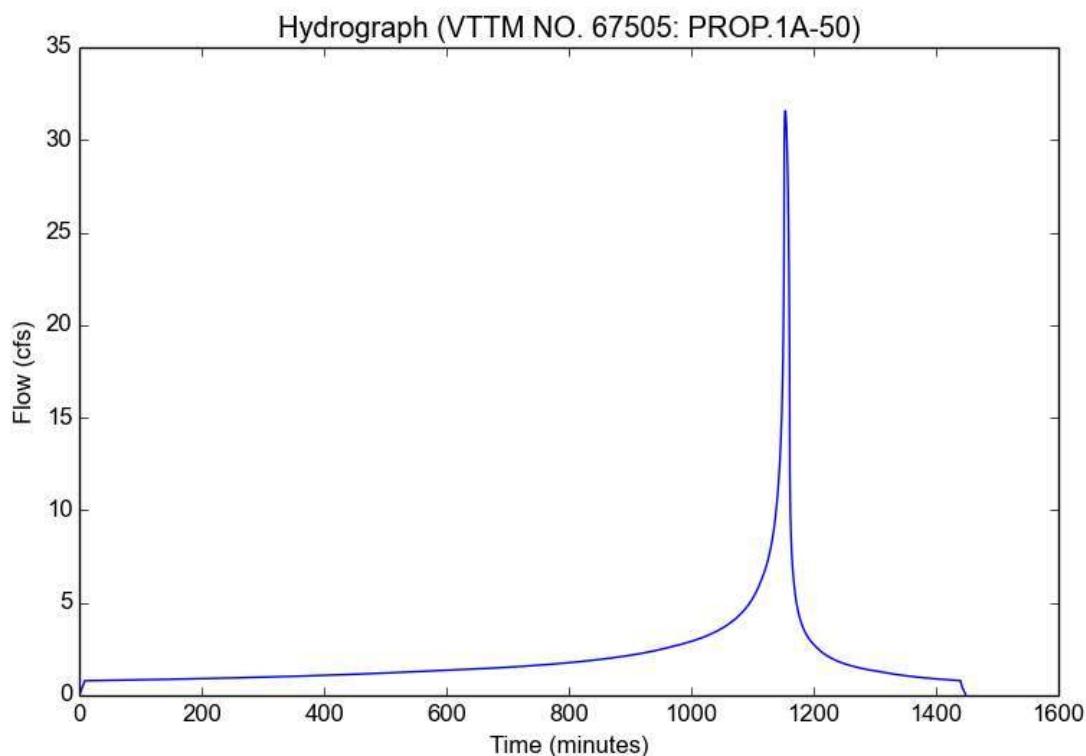
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM NO. 67505
Subarea ID	PROP.1A-50
Area (ac)	10.1
Flow Path Length (ft)	1151.0
Flow Path Slope (vft/hft)	0.015
50-yr Rainfall Depth (in)	7.7
Percent Impervious	0.35
Soil Type	2
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	7.7
Peak Intensity (in/hr)	3.4851
Undeveloped Runoff Coefficient (Cu)	0.8954
Developed Runoff Coefficient (Cd)	0.897
Time of Concentration (min)	9.0
Clear Peak Flow Rate (cfs)	31.574
Burned Peak Flow Rate (cfs)	31.574
24-Hr Clear Runoff Volume (ac-ft)	4.0533
24-Hr Clear Runoff Volume (cu-ft)	176560.9734



Section 3.0

WATER QUALITY CALCULATIONS

Peak Flow Hydrologic Analysis

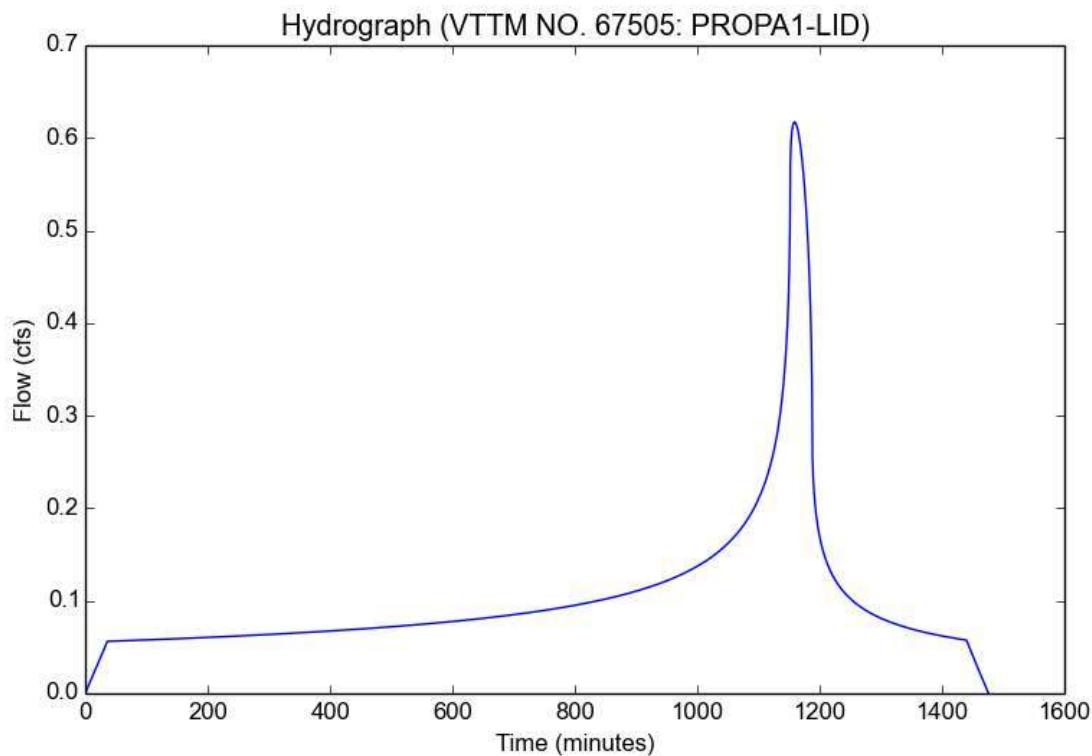
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM NO. 67505
Subarea ID	PROPA1-LID
Area (ac)	2.8
Flow Path Length (ft)	1151.0
Flow Path Slope (vft/hft)	0.015
85th Percentile Rainfall Depth (in)	1.1
Percent Impervious	0.9
Soil Type	2
Design Storm Frequency	85th percentile storm
Fire Factor	0
LID	True

Output Results

Modeled (85th percentile storm) Rainfall Depth (in)	1.1
Peak Intensity (in/hr)	0.2595
Undeveloped Runoff Coefficient (Cu)	0.3936
Developed Runoff Coefficient (Cd)	0.8494
Time of Concentration (min)	36.0
Clear Peak Flow Rate (cfs)	0.6172
Burned Peak Flow Rate (cfs)	0.6172
24-Hr Clear Runoff Volume (ac-ft)	0.2097
24-Hr Clear Runoff Volume (cu-ft)	9133.4156

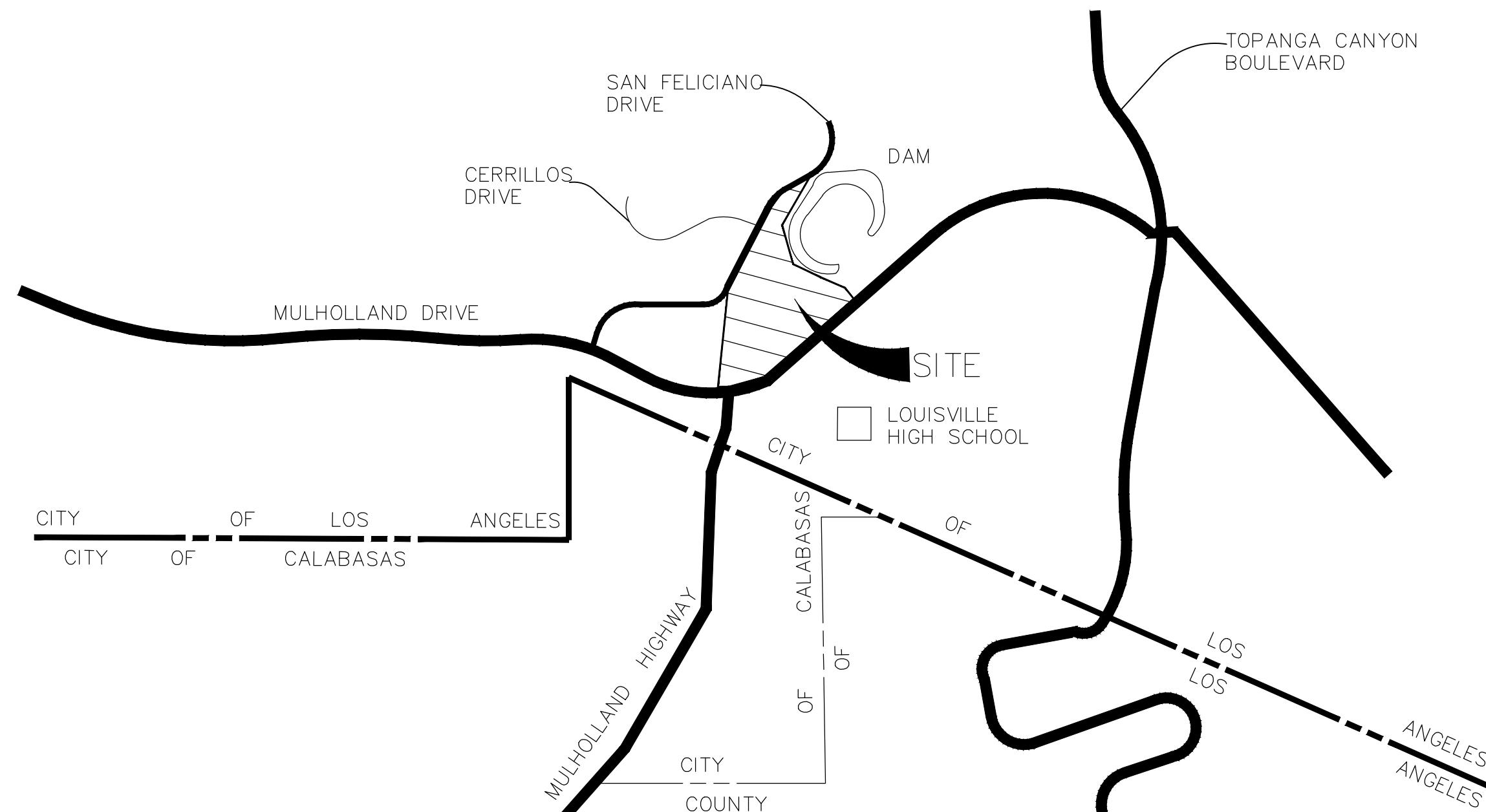


Section 4.0

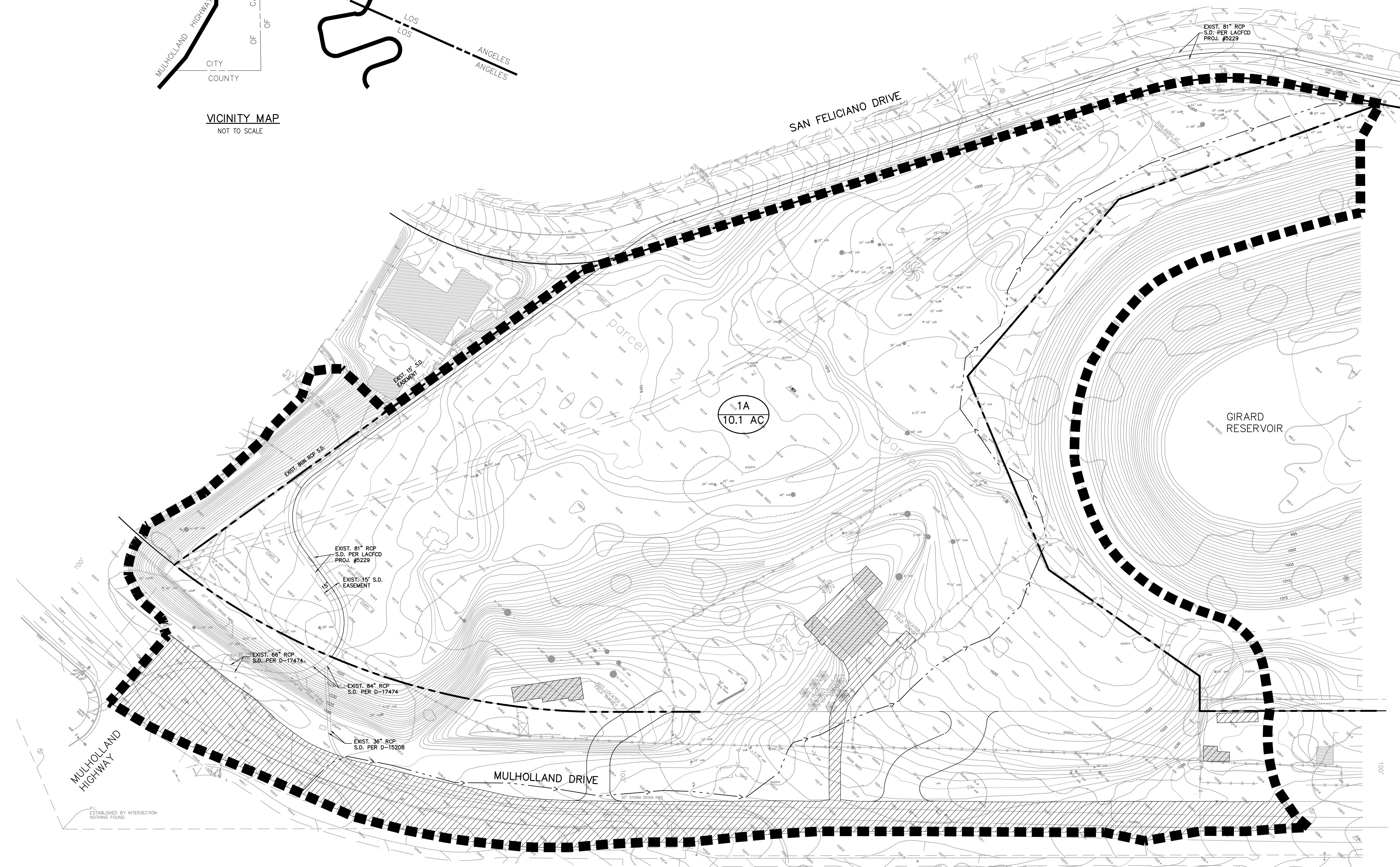
EXHIBITS (EXISTING & PROPOSED DRAINAGE MAPS)

VESTING TENTATIVE TRACT MAP NO. 67505

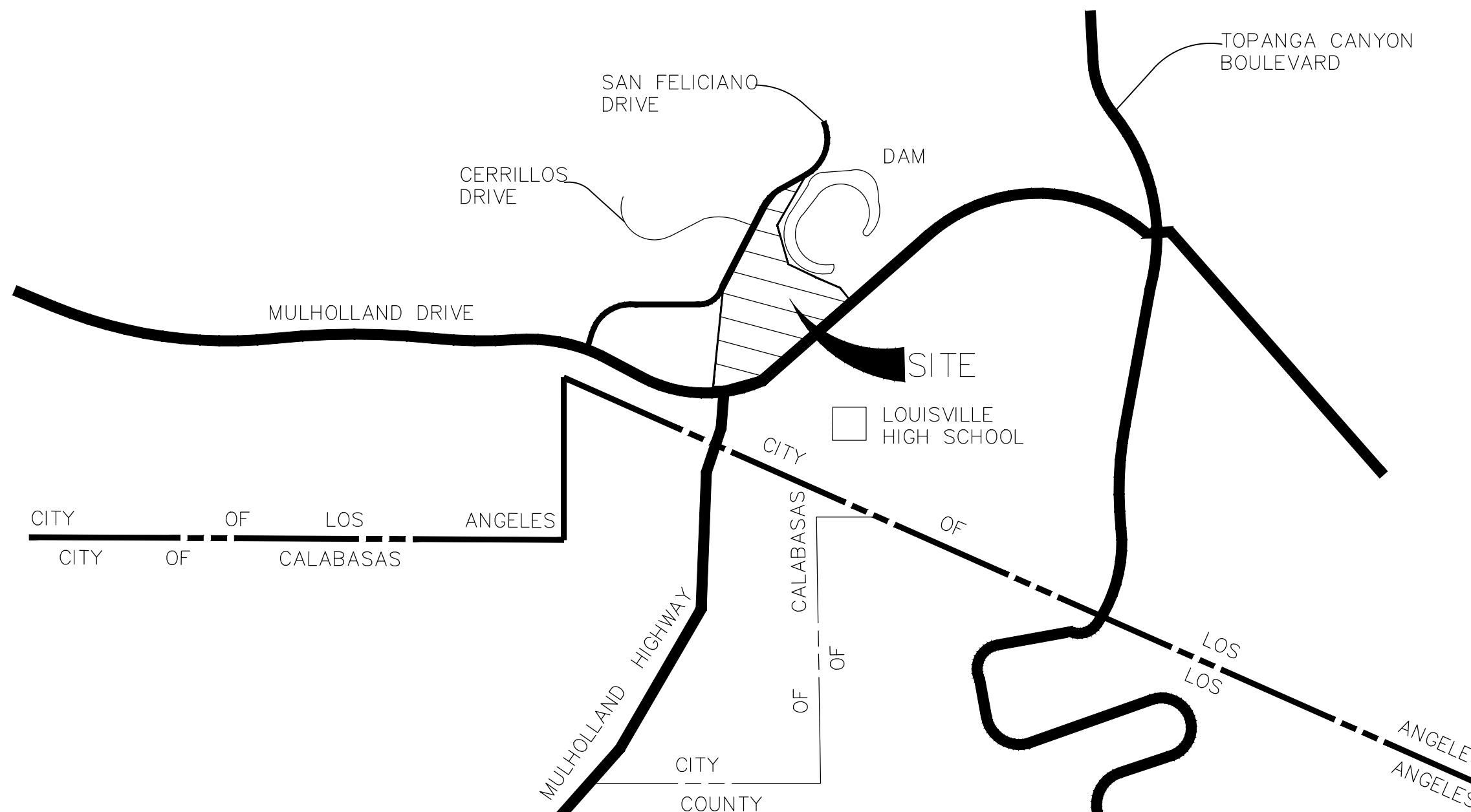
22241–22255 MULHOLLAND DRIVE



VICINITY MAP
NOT TO SCALE

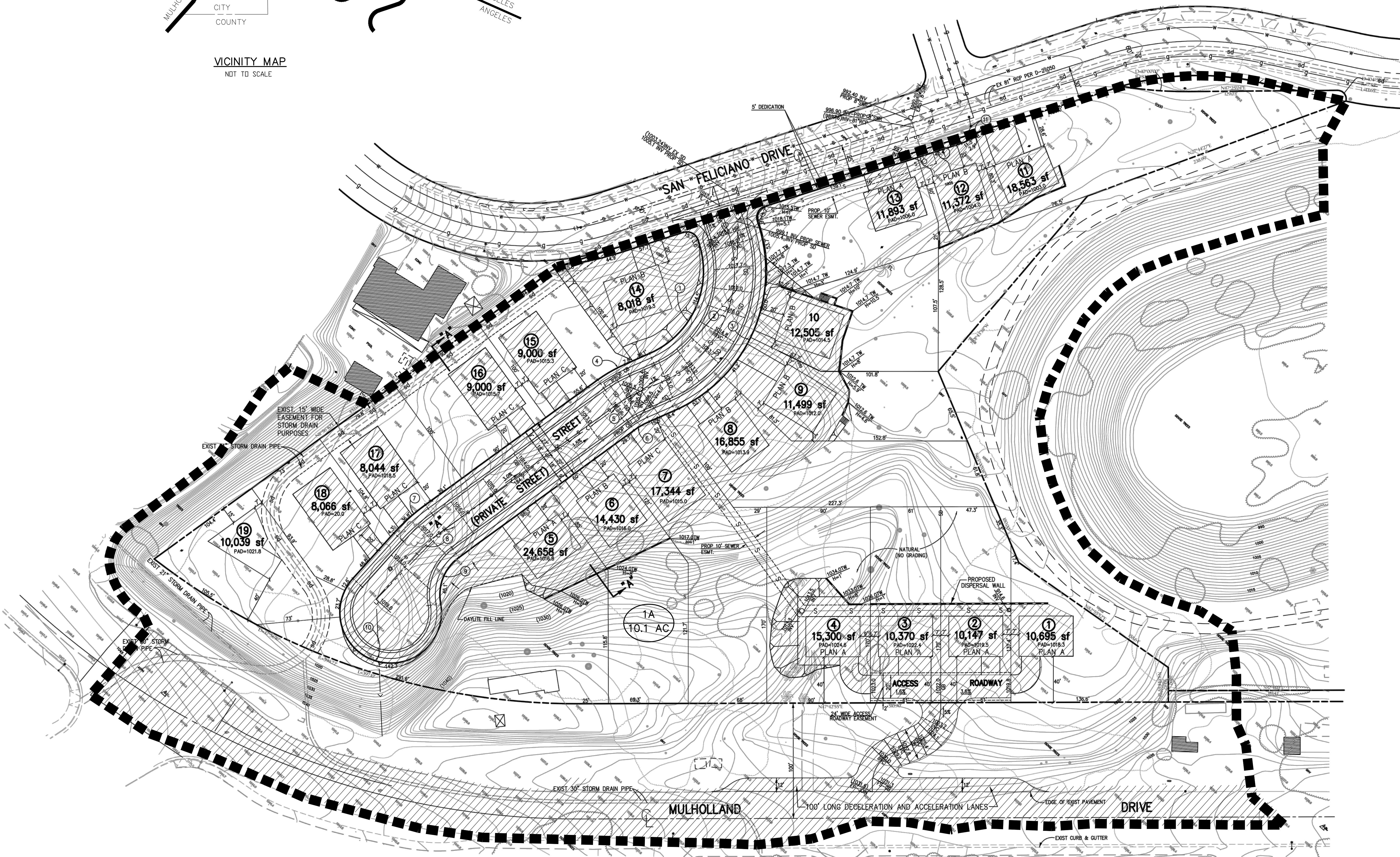


VESTING TENTATIVE TRACT MAP NO. 67505
22241-22255 MULHOLLAND DRIVE



VICINITY MAP

NOT TO SCALE



LEGEND / HYDROLOGIC DATA

- PROPERTY LINE

DRAINAGE SUBAREA BOUNDARY

AREA DESIGNATION AND ACREAGE

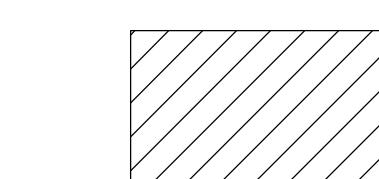
DRAINAGE FLOW PATH

STORM FREQUENCY: 25 YEARS, 50 YEARS

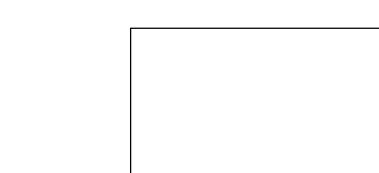
PREDOMINANT SOIL TYPE: 002

LAND USE: RESIDENTIAL

RAINFALL: 7.7 INCHES



IMPERVIOUS AREA



OPEN SPACE

% IMPERVIOUS CALCULATIONS			
SUBAREA	DESCRIPTION	AREA (AC)	% IMPERVIOUS
1A	OPEN SPACE	7.3	15
	IMPERVIOUS	2.8	90

PROPOSED HYDROLOGY EXHIBIT

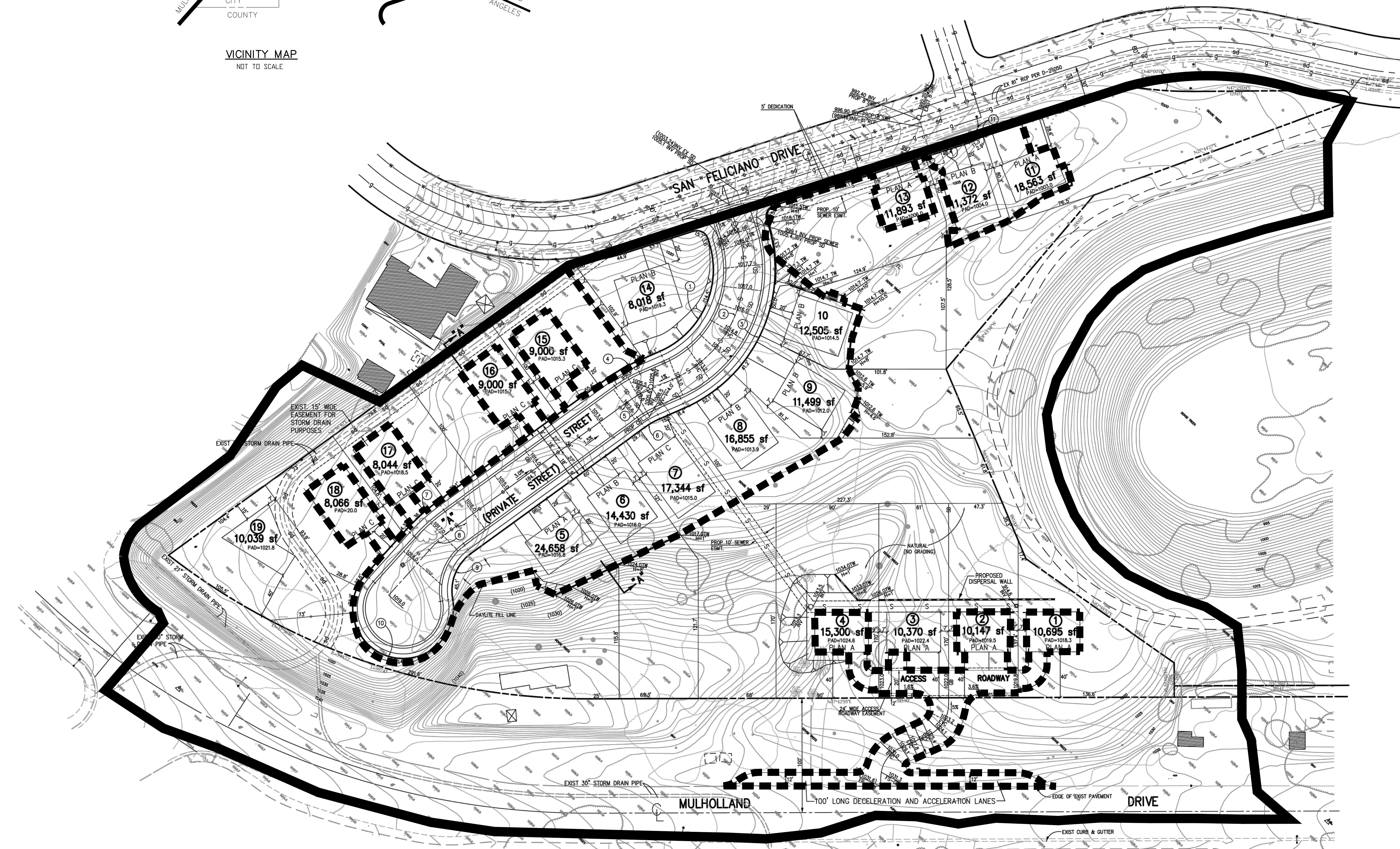
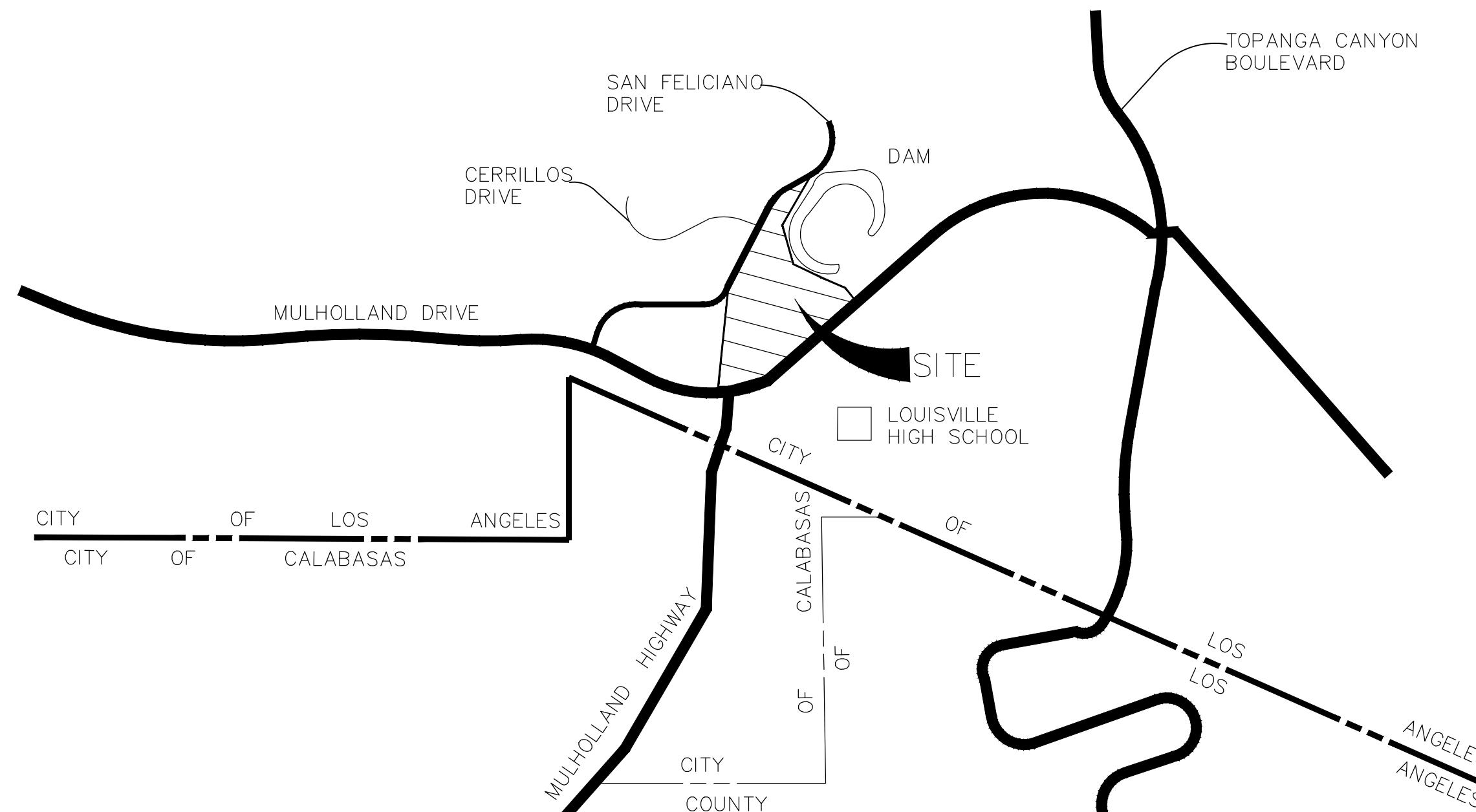
P S O M A S

1 A 3

HARRIDGE SAN FELICIANO LLC

VESTING TENTATIVE TRACT MAP NO. 67505

22241–22255 MULHOLLAND DRIVE

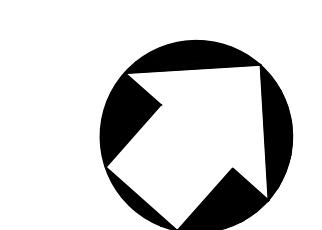


LEGEND / HYDROLOGIC DATA

- PROPERTY LINE
- DRAINAGE AREA BOUNDARY
- LID BOUNDARY
- AREA DESIGNATION AND ACREAGE
- DRAINAGE FLOW PATH
- STORM FREQUENCY: 85TH PERCENTILE STORM
- PREDOMINANT SOIL TYPE: 002
- LAND USE: RESIDENTIAL
- RAINFALL: 1.1 INCHES

% IMPERVIOUS CALCULATIONS			
SUBAREA	DESCRIPTION	AREA (AC)	% IMPERVIOUS
1A	IMPERVIOUS	2.80	90

PROPOSED
LID EXHIBIT



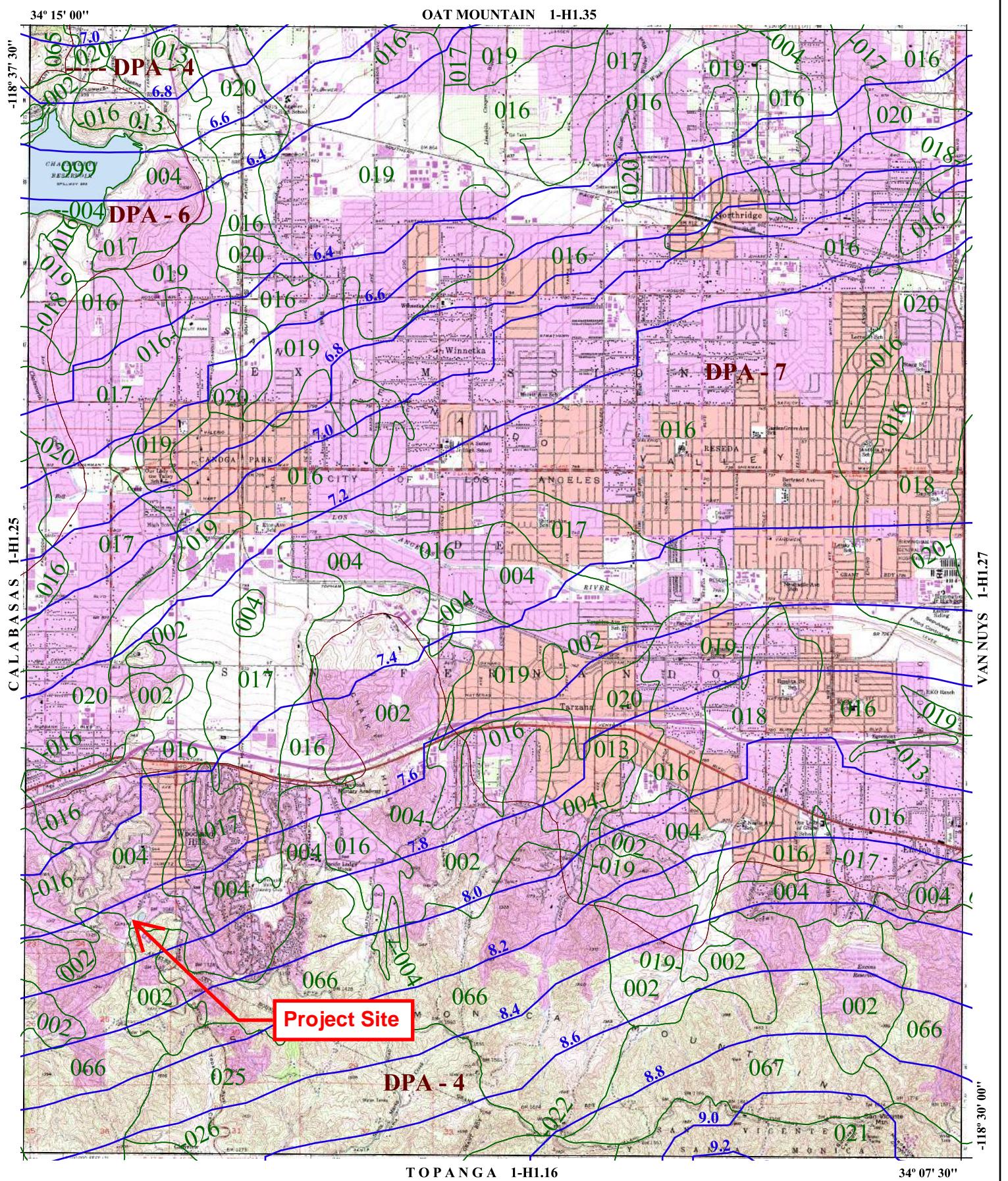
40 20 0' 40 80
GRAPHIC SCALE: 1'=40'
Note: For reduced sized prints, original scale is in inches

HARRIDGE SAN FELICIANO LLC

DATE: 09-28-18 REVISED ON:
JOB No:1HAR262500 SHEET 1 OF 1

Appendix

REFERENCE MATERIALS



25-YEAR 24-HOUR ISOHYET REDUCTION FACTOR: 0.878
10-YEAR 24-HOUR ISOHYET REDUCTION FACTOR: 0.714

CANOGA PARK
50-YEAR 24-HOUR ISOHYET

1-H1.26



1
0
1
2 Miles

